GEOTECHNICS PTY LTD
CONSULTING GEOTECHNICAL ENGINEERS

GEOTECHNICAL INVESTIGATION

FOR

NSW LAND AND HOUSING CORPORATION

5 – 9 Alexander Street, Fairy Meadow, New South Wales (BGXLP)

Report No: 22/0959

Project No: 31683/6213D-G

March 2022

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1. INTRODUCTION

This report presents the results of a Geotechnical Investigation carried out by STS Geotechnics Pty Limited (STS) for a proposed new residential development to be constructed at 5 to 9 Alexander Street, Fairy Meadow, NSW. At the time of writing this report STS were not provided with architectural drawings for the project. The report has been prepared assuming site development will be limited to one and two storey residential buildings without basement excavation.

The purpose of the investigation was to provide information on:

- Site conditions and regional geology,
- Subsurface conditions including groundwater levels (if encountered),
- Site Classification according to AS2870 (soil reactivity),
- Foundation design parameters including foundation options, and
- Exposure classification/soil aggressiveness according to AS2870.

The investigation was undertaken in accordance with STS proposal P22-146 dated March 10, 2022.

Our scope of work did not include a contamination assessment.

2. NATURE OF THE INVESTIGATION

2.1. Fieldwork

The fieldwork consisted of drilling five (5) boreholes numbered BH1 to BH5 (inclusive), at the locations shown on attached Drawing No. 22/0959. All boreholes were drilled using a utility mounted drilling rig, owned, and operated by STS. The soil strengths were assessed by carrying out a Dynamic Cone Penetrometer (DCP) tests adjacent to each borehole location.

Drilling operations were undertaken by one of STS's senior technical officers who also logged the subsurface conditions encountered.

Representative soil samples were collected from the boreholes for subsequent laboratory testing.

2.2. Laboratory Testing

To assess the soils for their aggressiveness, selected representative soil samples were tested to determine the following:

- pH,
- Sulphate content (SO_4),
- Chloride (Cl)
- Electrical Conductivity (EC), and

To assist with determining the site classification, three Shrink Swell test was carried out on a representative sample retrieved from the site.

Detailed test reports are given in Appendix B.

3. GEOLOGY AND SITE CONDITIONS

The Wollongong-Port Hacking geological map at a scale of 1:100,000 shows the site is underlain by Quaternary age soils comprising quartz and lithic fluvial sand, silt and clay.

The site is trapezoidal in shape and about 2350 m^2 in area. At the time of the fieldwork, the site was vacant, previous dwellings having been demolished. Site vegetation comprises trees and some grass. The ground surface falls about 0.5 meters to the east.

The site is bound by Alexander Street to the west and other residential sites.

4. SUBSURFACE CONDITIONS

When assessing the subsurface conditions across a site from a limited number of boreholes, there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour regarding the proposed development. The actual condition at the site may differ from those inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies, particularly on a site such as this where there has been previous development.

The subsurface conditions generally consist of fill overlying silty clays. The fill is 0.15 to 0.3 metres thick. Soft becoming very stiff silty clays underlie the fill, to the depth of drilling, 3.0 metres.

Groundwater was not observed during drilling works, however, moist to wet soils were noted in some of the boreholes.

The subsurface conditions observed are recorded on the borehole logs given in Appendix A. An explanation of the terms used on the logs is also given in Appendix A. Notes relating to geotechnical reports are also attached.

5. GEOTECHNICAL DISCUSSION

5.1. Site Classification (AS2870)

The classification has been prepared in accordance with the guidelines set out in the "Residential Slabs and Footings" Code, AS2870 – 2011.

To assist with determining the site classification, four shrink/swell tests were carried out on representative samples retrieved from the site. The detailed test report is attached and summarised in Table 5.1 below:

Table 5.1 Shrink Swell result summary

Location	Depth (m)	Material Description	Shrink/Swell Index (% per ΔpF)
BH1	0.8 – 1.0	Yellow brown silty clay	5.3
BH5	1.2 – 1.4	Orange brown silty clay	6.7

The sample collected from BH3 was unsuitable to carryout shrink swell testing. Experience has shown that at times the shrink swell index can be estimated by dividing the Plasticity Index (PI) by a factor of 10. The PI for this sample was 57% which implies the shrink swell index is 5.7% per ΔpF. This is consistent with the values reports above.

Because there are trees and the presence of previous dwellings, abnormal moisture conditions (AMC) prevail at the site. (Refer to Section 1.3.3 of AS2870).

Because of the AMC present and low strength soils, the site is classified a *Problem Site (P).* Because of the low strength soils, it is inappropriate to reclassify the site,

Foundation design and construction consistent with this classification shall be adopted as specified in the above referenced standard and in accordance with the following design details.

5.2. Foundation Design Parameters

We do not recommend founding any structural loads within the fill layer and low strength soils.

Pad and/or strip footings founded in the natural, firm silty clays, may be proportioned using an allowable bearing pressure of 75kPa. The minimum depth of founding must comply with the requirements of AS2870. To overcome the presence of the trees, the foundations should be designed in accordance with the procedures given in Appendices H and CH of AS2870-2011.

Piles founded in very stiff material may be proportioned using and allowable end bearing pressure of 300kPa, provided their depth to diameter ratio exceeds a value of 4. A shaft adhesion of 20 kPa may be adopted below a depth of 0.5 metres.

To ensure the bearing values given can be achieved, care should be taken to ensure the base of the excavations is free of all loose material prior to concreting. To this end, it is recommended that all excavations be concreted as soon as possible, preferably immediately after excavating, cleaning, inspecting and approval. Pier excavations should not be left open overnight. The possibility of groundwater inflow needs to be considered when drilling the piers and pouring concrete.

Because of the low strength soils, the site is considered unsuitable for slab on ground construction, piles will be required to suspend the slab. The slab may be proportioned for movements consistent with a Highly Reactive (H2) classification.

During foundation construction, should the subsurface conditions vary to those inferred in this report, a suitably experienced geotechnical engineer should review the design and recommendations given above to determine if any alterations are required.

5.3. Soil Aggressiveness

The aggressiveness or erosion potential of an environment in building materials, particularly concrete and steel is dependent on the levels of soil pH and the types of salts present, generally sulfates and chlorides. To determine the degree of aggressiveness, the test values obtained are compared to Tables 6.4.2 (C) and 6.5.2 (C) in AS2159 – 2009 Piling – Design and Installation. The test results are summarised in Table 5.2.

Table 5.2 – Soil Aggressiveness Summary

Sample No.	Location	Depth (m)	pH	Sulfate (mg/kg)	Electrical Conductivity (dS/m)	
					$EC_{1:5}$	EC_e
S1	BH1	0.4	5.2	50	0.065	0.6
S2	BH3	0.6	5.4	110	0.125	1.1
S3	BH5	1.0	4.9	80	0.158	1.4
S4	BH4	0.8	5.0	110	0.143	1.3

The soils samples were cohesive and above groundwater. Therefore, soil conditions B are considered appropriate (AS2159).

In accordance with AS2159-2009 the exposure classification for the onsite soils non-aggressive to steel and mildly aggressive to concrete. In accordance with AS2870-2011 the soils are classified as A2.

Reference to DLWC (2002) "Site Investigations for Urban Salinity" indicates that ECe values of 0.6 to 1.4 dS/m are consistent with the presence of non-saline soils.

6. FINAL COMMENTS

During construction, should the subsurface conditions vary from those inferred above, we would be contacted to determine if any changes should be made to our recommendations. The exposed bearing surfaces for footings should be inspected by a geotechnical engineer to ensure the allowable pressure given has been achieved.

Attention is drawn to Appendix B of AS2870 - 2011 regarding the need to properly maintain the foundations. Surface drainage should be provided to avoid the possibility of water ponding near the building and the finished ground surface should fall at least 50 mm over one metre away from the building.

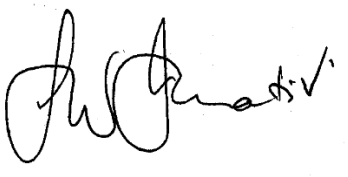
The above classification has been made assuming that all footings will bear in either natural ground or in controlled filling. Prior to the placement of any filling the existing surface should be stripped of all vegetation and topsoil.

If excavations for rainwater or detention tanks are to be made within 6 metres of the building foundations, advice should be sought regarding their effect on the foundations.

Placing absorption trenches on the high side of the property may create abnormal moisture conditions for the foundations (Refer to Section 1.3.3 of AS2870). This could have a negative effect on the foundation performance and more than likely alter the site classification provided above.

This report has been prepared assuming that no trees other than those noted will be present on the site. If future tree planting is planned, eg. there is a landscaping plan, their effect on the foundation performance must be considered.

This report has been prepared assuming the site development will be limited to one or two storey residential buildings. The information and interpretation may not be relevant if the design proposal changes (e.g. to a five-storey building involving major cuts during the site preparation). If changes occur, we would be pleased to review the report and advise on the adequacy of the investigation.



Laurie Ihnativ
Principal Geotechnical Engineer
STS Geotechnics *Pty Limited*

N

STS Geotechnics Pty. Ltd.	Scale: Unknown	Date: March 2022
Client: NSW LAND & HOUSING CORPORATION		
GEOTECHNICAL INVESTIGATION 5-9 ALEXANDER STREET, FAIRY MEADOW BOREHOLE AND PENETROMETER LOCATIONS		Project No. 31683/6213D-G Drawing No: 22/0959

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Tree Heights and Type

Project: 5-9 Alexander Street, Fairy Meadow — Project No. / STS No.: 31683/6213D-G

Client: NSW Land & Housing Corporation — Technician: MB

Tree No.	Canopy Radius (m)	Distance from Tree Along Ground (m)	Uphill / Level / Downhill	Height of Tree (m)	Native (Y / N)	Growing / Mature
T1	6			8	Unknown	M
T2	20			20	Unknown	M
T3	15			15	Unknown	M
T4	10			10	Unknown	M

NOTES RELATING TO GEOTECHNICAL REPORTS

Introduction

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report.

When copies of reports are made, they should be reproduced in full.

Geotechnical Reports

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by STS Geotechnics Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, STS Geotechnics Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

Unforeseen Conditions

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, STS Geotechnics Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

Subsurface Information

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

Supply of Geotechnical Information or Tendering Purposes

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.

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APPENDIX A – BOREHOLE LOGS AND EXPLANATION SHEETS

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GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project / STS No. 31683/6213D-G	**BOREHOLE NO.: BH 1**
Project: 5-9 Alexander Street, Fairy Meadow	Date: March 11, 2022	
Location: Refer to Drawing No. 22/0959	Logged: MB Checked By: IW	Sheet 1 of 1

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
			FILL: SILTY GRAVELLY CLAY: brown, low to medium plasticity	CL	-	D-M
			SILTY CLAY: grey brown, trace of orange brown, medium plasticity	CL	FIRM TO STIFF	D-M
	S1 @ 0.4 m	0.5				
			SILTY CLAY: grey with orange brown, medium to high plasticity	CL/CH	FIRM TO STIFF	D-M
		1.0				
	U50					
		1.5			VERY STIFF	
		2.0				
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (°): 0
Drill Bit: Spiral

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

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GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project / STS No. 31683/6213D-G	**BOREHOLE NO.: BH 2**
Project: 5-9 Alexander Street, Fairy Meadow	Date: March 11, 2022	
Location: Refer to Drawing No. 22/0959	Logged: MB Checked By: IW	Sheet 1 of 1

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
			FILL: SILTY GRAVELLY CLAY: brown, medium plasticity	CL	-	M-W
			SILTY CLAY: grey with orange brown, medium to high plasticity	CL/CH	FIRM	M-W
		0.5			FIRM TO STIFF	
					VERY STIFF	
		1.0				
		1.5				
		2.0				
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample

WT - level of water table or free water N - Standard Penetration Test (SPT)

S - jar sample

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

Contractor: STS

Equipment: Christie

Hole Diameter (mm): 100

Angle from Vertical (°): 0

Drill Bit: Spiral

STS Geotechnics Pty Ltd

GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project / STS No. 31683/6213D-G	**BOREHOLE NO.:**	**BH 3**
Project: 5-9 Alexander Street, Fairy Meadow	Date: March 11, 2022		
Location: Refer to Drawing No. 22/0959	Logged: MB Checked By: IW	Sheet 1 of 1	

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
			FILL: SILTY GRAVELLY CLAY: brown, medium plasticity	CL	-	W
			SILTY CLAY: grey with orange brown, medium to high plasticity	CL/CH	FIRM	M
	S2 @ 0.6 m	0.5				
		1.0				
	U50				STIFF	
		1.5			VERY STIFF	
		2.0				
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (°): 0
Drill Bit: Spiral

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GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project / STS No. 31683/6213D-G	**BOREHOLE NO.: BH 4**
Project: 5-9 Alexander Street, Fairy Meadow	Date: March 11, 2022	
Location: Refer to Drawing No. 22/0959	Logged: MB Checked By: IW	Sheet 1 of 1

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
			FILL: SILTY GRAVELLY CLAY: brown, medium plasticity	CL	-	M-W
			SILTY CLAY: grey with orange brown, medium to high plasticity	CL/CH	FIRM	M-W
		0.5			STIFF	
	S4 @ 0.8 m				VERY STIFF	
		1.0				
		1.5				
		2.0				
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (°): 0
Drill Bit: Spiral

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GEOTECHNICAL LOG - NON CORE BOREHOLE

Client: NSW Land & Housing Corporation	Project / STS No. 31683/6213D-G	**BOREHOLE NO.:**	**BH 5**
Project: 5-9 Alexander Street, Fairy Meadow	Date: March 11, 2022		
Location: Refer to Drawing No. 22/0959	Logged: MB Checked By: IW	Sheet 1 of 1	

WATER TABLE	SAMPLES	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	SYMBOL	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	MOISTURE
			FILL: SILTY GRAVELLY CLAY: brown, medium plasticity	CL	-	M-W
			SILTY CLAY: grey with orange brown, medium to high plasticity	CL/CH	SOFT	M-W
		0.5			FIRM	
	S3 @ 1.0 m	1.0				
	U50					
		1.5			VERY STIFF	
		2.0				
		2.5				
			BOREHOLE DISCONTINUED AT 3.0 M			

D - disturbed sample U - undisturbed tube sample B - bulk sample
WT - level of water table or free water N - Standard Penetration Test (SPT)
S - jar sample

NOTES: See explanation sheets for meaning of all descriptive terms and symbols

Contractor: STS
Equipment: Christie
Hole Diameter (mm): 100
Angle from Vertical (°): 0
Drill Bit: Spiral

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Dynamic Cone Penetrometer Test Report

Project: 5-9 ALEXANDER STREET, FAIRY MEADOW
Client: NSW LAND & HOUSING CORPORATION
Address: 31-39 Macquarie Street, Parramatta
Test Method: AS 1289.6.3.2

Project No.: 31683/6213D-G
Report No.: 22/0958
Report Date: 16/03/2022
Page: 1 of 1

NATA
Accredited for compliance with ISO/IEC 17025 - Testing
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards
NATA Accreditation Number 2750

Site No.	P1	P2	P3	P4	P5	
Location	Refer to Drawing No. 22/0959	Refer to Drawing No. 22/0959	Refer to Drawing No. 22/0959	Refer to Drawing No. 22/0959	Refer to Drawing No. 22/0959	
Date Tested	11/3/2022	11/3/2022	11/3/2022	11/3/2022	11/3/2022	
Starting Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level	
Depth (m)	Penetration Resistance (blows / 150mm)					
0.00 - 0.15	2	1	1	2	2	
0.15 - 0.30	3	2	1	2	2	
0.30 - 0.45	3	2	3	2	1	
0.45 - 0.60	3	4	3	4	1	
0.60 - 0.75	2	4	3	6	2	
0.75 - 0.90	2	7	3	8	3	
0.90 - 1.05	3	7	2	8	3	
1.05 - 1.20	3	10	2	8	2	
1.20 - 1.35	3	13	2	10	2	
1.35 - 1.50	4	16	4	16	2	
1.50 - 1.65	6	23+	8	23+	7	
1.65 - 1.80	12	Refusal	10	Refusal	8	
1.80 - 1.95	18		10		10	
1.95 - 2.10	23+		17		10	
2.10 - 2.25	Refusal		23+		16	
2.25 - 2.40			Refusal		23+	
2.40 - 2.55					Refusal	
2.55 - 2.70						
2.70 - 2.85						
2.85 - 3.00						
3.00 - 3.15						
3.15 - 3.30						
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						

Remarks: * Pre drilled prior to testing

Approved Signatory.....
Orlando Mendoza - Laboratory Manager

Technician: MB

E1. CLASSIFICATION OF SOILS

E1.1 Soil Classification and the Unified System

An assessment of the site conditions usually includes an appraisal of the data available by combining values of engineering properties obtained by the site investigation with descriptions, from visual observation of the materials present on site.

The system used by STS Geotechnics Pty Ltd (STS) in the identification of soil is the Unified Soil Classification system (USC) which was developed by the US Army Corps of Engineers during World War II and has since gained international acceptance and has been adopted in its metricated form by the Standards Association of Australia.

The Australian Site Investigation Code (AS1726-2017, Appendix D) recommends that the description of a soil includes the USC group symbols which are an integral component of the system.

The soil description should contain the following information in order:

Soil composition

- SOIL NAME and USC classification symbol (IN BLOCK LETTERS)
- plasticity or particle characteristics
- colour
- secondary and minor constituents (name estimated proportion, plasticity or particle characteristics, colour

Soil condition

- moisture condition
- consistency or density index

Soil structure

- structure (zoning, defects, cementing)

Soil origin

interpretation based on observation eg FILL, TOPSOIL, RESIDUAL, ALLUVIUM.

E1.2 Soil Composition

(a) Soil Name and Classification Symbol

The USC system is summarised in Figure E1.2.1. The primary division separates soil types on the basis of particle size into:

- Coarse grained soils - more than 50% of the material less than 60 mm is larger than 0.06 mm (60 µm).

- Fine grained soils - more than 50% of the material less than 60 mm is smaller than 0.06 mm (60 µm).

Initial classification is by particle size as shown in Table E1.2.1. Further classification of fine grained soils is based on plasticity.

TABLE E1.2.1 - CLASSIFICATION BY PARTICLE SIZE

NAME	SUB-DIVISION	SIZE
Clay (1)		< 2 µm
Silt (2)		2 µm to 60 µm
Sand	Fine Medium Coarse	60 µm to 200 µm 200 µm to 600 µm 600 µm to 2 mm
Gravel (3)	Fine Medium Coarse	2 mm to 6 mm 6 mm to 20 mm 20 mm to 60 mm
Cobbles (3)		60 mm to 200 mm
Boulders (3)		> 200 mm

Where a soil contains an appropriate amount of secondary material, the name includes each of the secondary components (greater than 12%) in increasing order of significance, eg sandy silty clay.

Minor components of a soil are included in the description by means of the terms “some” and “trace” as defined in Table E1.2.2.

TABLE E1.2.2 - MINOR SOIL COMPONENTS

TERM	DESCRIPTION	APPROXIMATE PROPORTION (%)
Trace	presence just detectable, little or no influence on soil properties	0-5
Some	presence easily detectable, little influence on soil properties	5-12

The USC group symbols should be included with each soil description as shown in Table E1.2.3

TABLE E1.2.3 - SOIL GROUP SYMBOLS

SOIL TYPE	PREFIX
Gravel	G
Sand	S
Silt	M
Clay	C
Organic	O
Peat	Pt

The group symbols are combined with qualifiers which indicate grading, plasticity or secondary components as shown on Table E1.2.4

TABLE E1.2.4 - SOIL GROUP QUALIFIERS

SUBGROUP	SUFFIX
Well graded	W
Poorly Graded	P
Silty	M
Clayey	C
Liquid Limit <50% - low to medium plasticity	L
Liquid Limit >50% - medium to high plasticity	H

(b) Grading

"Well graded" Good representation of all particle sizes from the largest to the smallest.

"Poorly graded" One or more intermediate sizes poorly represented

"Gap graded" One or more intermediate sizes absent

"Uniformly graded" Essentially single size material.

(c) Particle shape and texture

The shape and surface texture of the coarse grained particles should be described.

Angularity may be expressed as "rounded", "sub-rounded", "sub-angular" or "angular".

Particle **form** can be "equidimensional", "flat" or elongate".

Surface texture can be "glassy", "smooth", "rough", pitted" or striated".

(d) Colour

The colour of the soil should be described in the moist condition using simple terms such as:

Black White Grey Red
Brown Orange Yellow Green
Blue

These may be modified as necessary by "light" or "dark". Borderline colours may be described as a combination of two colours, eg red-brown.

For soils that contain more than one colour terms such as:

- Speckled Very small (<10 mm dia) patches
- Mottled Irregular
- Blotched Large irregular (>75 mm dia)
- Streaked Randomly oriented streaks

(e) Minor Components

Secondary and minor components should be individually described in a similar manner to the dominant component.

*E1.3 **S**oil Condition*

(a) Moisture

Soil moisture condition is described as "dry", "moist" or "wet".

The moisture categories are defined as:
Dry (D) - Little or no moisture evident. Soils are running.
Moist (M) - Darkened in colour with cool feel. Granular soil particles tend to adhere. No free water evident upon remoulding of cohesive soils.

In addition the moisture content of cohesive soils can be estimated in relation to their liquid or plastic limit.

(b) Consistency

Estimates of the consistency of a clay or silt soil may be made from manual examination, hand penetrometer test, SPT results or from laboratory tests to determine undrained shear or unconfined compressive strengths. The classification of consistency is defined in Table E1.3.1.

TABLE E1.3.1 - CONSISTENCY OF FINE-GRAINED SOILS

TERM	UNCONFINED STRENGTH (kPa)	FIELD IDENTIFICATION
Very Soft	<25	Easily penetrated by fist. Sample exudes between fingers when squeezed in the fist.
Soft	25 - 50	Easily moulded in fingers. Easily penetrated 50 mm by thumb.
Firm	50 - 100	Can be moulded by strong pressure in the fingers. Penetrated only with great effort.
Stiff	100 - 200	Cannot be moulded in fingers. Indented by thumb but penetrated only with great effort.
Very Stiff	200 - 400	Very tough. Difficult to cut with knife. Readily indented with thumb nail.
Hard	>400	Brittle, can just be scratched with thumb nail. Tends to break into fragments.

Unconfined compressive strength as derived by a hand penetrometer can be taken as approximately double the undrained shear strength ($q_u = 2\ c_u$).

(c) Density Index

The insitu density index of granular soils can be assessed from the results of SPT or cone penetrometer tests. Density index should not be estimated visually.

TABLE E1.3.2 - DENSITY OF GRANULAR SOILS

TERM	SPT N VALUE	STATIC CONE VALUE q_c (MPa)	DENSITY INDEX (%)
Very Loose	0 - 3	0 - 2	0 - 15
Loose	3 - 8	2 - 5	15 - 35
Medium Dense	8 - 25	5 - 15	35 - 65
Dense	25 - 42	15 - 20	65 - 85
Very Dense	>42	>20	>85

E1.4 Soil Structure

(a) Zoning

A sample may consist of several zones differing in colour, grain size or other properties. Terms to classify these zones are:

Layer - continuous across exposure or sample
Lens - discontinuous with lenticular shape
Pocket - irregular inclusion
Each zone should be described, their distinguishing features, and the nature of the interzone boundaries.

(b) Defects

Defects which are present in the sample can include:

- fissures
- roots (containing organic matter)
- tubes (hollow)
- casts (infilled)

Defects should be described giving details of dimensions and frequency. Fissure orientation, planarity, surface condition and infilling should be noted. If there is a tendency to break into blocks, block dimensions should be recorded

E1.5 Soil Origin

Information which may be interpretative but which may contribute to the usefulness of the material description should be included. The most common interpreted feature is the origin of the soil. The assessment of the probable origin is based on the soil material description, soil structure and its relationship to other soil and rock materials.

Common terms used are:

“Residual Soil” - Material which appears to have been derived by weathering from the underlying rock. There is no evidence of transport.

“Colluvium” - Material which appears to have been transported from its original location. The method of movement is usually the combination of gravity and erosion.

“Landslide Debris” - An extreme form of colluvium where the soil has been transported by mass movement. The material is obviously distributed and contains distinct defects related to the slope failure.

“Alluvium” - Material which has been transported essentially by water. usually associated with former stream activity.

“Fill” - Material which has been transported and placed by man. This can range from natural soils which have been placed in a controlled manner in engineering construction to dumped waste material. A description of the constituents should include an assessment of the method of placement.

E1.6 Fine Grained Soils

The physical properties of fine grained soils are dominated by silts and clays.

The definition of clay and silt soils is governed by their Atterberg Limits. Clay soils are characterised by the properties of cohesion and plasticity with cohesion defines as the ability to deform without rupture. Silts exhibit cohesion but have low plasticity or are non-plastic.

The field characteristics of clay soils include:

- dry lumps have appreciable dry strength and cannot be powdered
- volume changes occur with moisture content variation
- feels smooth when moist with a greasy appearance when cut.

The field characteristics of silt soils include:

- dry lumps have negligible dry strength and can be powdered easily
- dilatancy - an increase in volume due to shearing - is indicted by the presence of a shiny film of water after a hand sample is shaken. The water disappears upon remoulding. Very fine grained sands may also exhibit dilatancy.
- low plasticity index
- feels gritty to the teeth

E1.7 Organic Soils

Organic soils are distinguished from other soils by their appreciable content of vegetable matter, usually derived from plant remains.

The soil usually has a distinctive smell and low bulk density.

The USC system uses the symbol Pt for partly decomposed organic material. The O symbol is combined with suffixes “O” or “H” depending on plasticity.

Where roots or root fibres are present their frequency and the depth to which they are encountered should be recorded. The presence of roots or root fibres does not necessarily mean the material is an “organic material” by classification.

Coal and lignite should be described as such and not simply as organic matter.

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APPENDIX B – LABORATORY TEST RESULTS

STS GEOTECHNICS PTY LTD
CONSULTING GEOTECHNICAL ENGINEERS

STS Geotechnics Pty Ltd
14/1 Cowpasture Place, Wetherill Park NSW 2164
Phone: (02)9756 2166 | Email: enquiries@stsgeo.com.au

Shrink Swell Index Report

Project: 5 - 9 Alexander Street, Fairy Medow
Client: NSW Land and Housing Corporation
Address: 31-39 Macquarie Street, Parramatta
Test Method: AS1289.7.1.1

Project No.: 31683/6213D-L
Report No.: 22/0983
Report Date: 17/03/2022
Page: 1 of 1

Sampling Procedure: AS 1289.1.3.1 Clause 3.1.3.2 - Thin Walled Sampler

STS / Sample No.		6213D-L/1	6213D-L/3				
Sample Location		Borehole 1	Borehole 5				
Material Description		Silty Clay, yellow brown	Silty Clay, orange brown				
Depth (m)		0.8 - 1.0	1.2 - 1.4				
Sample Date		14/03/2022	14/03/2022				
Shrink	Moisture Content (%)	44.2	43.1				
	Soil Crumbling	NA	NA				
	Extent of Cracking	Open	Open				
	Strain (%)	9.1	10.7				
Swell	Moisture Content Initial (%)	39.7	40.0				
	Moisture Content Final (%)	42.9	45.5				
	Strain (%)	1.0	2.7				
Inert Inclusions (%)		<15	<10				
Shrink Swell Index (%)		**5.3**	**6.7**				

Remarks:

Approved Signatory.....

Technician: DH

Philip Ihnativ - Senior Geotechnician

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STS Geotechnics Pty Ltd
14/1 Cowpasture Place, Wetherill Park NSW 2164
Phone: (02)9756 2166 | Email: enquiries@stsgeo.com.au

NATA Accredited for Compliance with ISO/IEC 17025 - Testing No. 2750

Atterberg Limits and Linear Shrinkage Report

Project: 5 - 9 Alexander Street, Fairy Meadow — Project No.: 31683

Client: NSW Land and Housing Corporation — Report No.: 22/0997

Address: 31-39 Macquarie Street, Parramatta — Report Date: 18/03/2022

Test Method: AS1289.3.1.2,3.2.1,3.1.1,3.4.1,2.1.1 — Page: 1 of 1

Sampling Procedure: AS 1289.1.2.1 Clause 6.5.3 - Power Auger Drilling (Not covered under NATA Scope of Accreditation)

STS / Sample No.	6213D-L/1					
Sample Location	Borehole 3 Refer to Drawing No. 22/0959					
Material Description	Gravelly Clay, yellow grey brown					
Depth (m)	1.2 - 1.4					
Sample Date	11/03/2022					
Sample History	Oven Dried					
Method of Preparation	Dry Sieved					
Liquid Limit (%)	**84**					
Plastic Limit (%)	**27**					
Plasticity Index	**57**					
Linear Shrinkage (%)	**18.5**					
Mould Size (mm)	127					
Crumbing	N					
Curling	N					

Remarks:

Approved Signatory.....

Technician: DH

Orlando Mendoza - Laboratory Manager

ALS Environmental

CERTIFICATE OF ANALYSIS

Work Order	: **ES2208971**	Page	: 1 of 4
Client	: **STS Geotechnics**	Laboratory	: Environmental Division Sydney
Contact	: ENQUIRES STS	Contact	: Customer Services ES
Address	: Unit 14/1 Cowpasture Place Wetherill Park 2164	Address	: 277-289 Woodpark Road Smithfield NSW Australia 2164
Telephone	: ----	Telephone	: +61-2-8784 8555
Project	: 30055/30060/31683	Date Samples Received	: 14-Mar-2022 17:30
Order number	: 2022-082	Date Analysis Commenced	: 15-Mar-2022
C-O-C number	: ----	Issue Date	: 17-Mar-2022 20:00
Sampler	: MB		
Site	: ----		
Quote number	: EN/222		
No. of samples received	: 7		
No. of samples analysed	: 7		

Accreditation No. 825
Accredited for compliance with
ISO/IEC 17025 - Testing

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted, unless the sampling was conducted by ALS. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

Signatories	*Position*	*Accreditation Category*
Ankit Joshi	Senior Chemist - Inorganics	Sydney Inorganics, Smithfield, NSW
Ivan Taylor	Analyst	Sydney Inorganics, Smithfield, NSW
Wisam Marassa	Inorganics Coordinator	Sydney Inorganics, Smithfield, NSW

General Comments

The analytical procedures used by ALS have been developed from established internationally recognised procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are fully validated and are often at the client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contact for details.

Key : CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.
LOR = Limit of reporting
^ = This result is computed from individual analyte detections at or above the level of reporting
ø = ALS is not NATA accredited for these tests.
~ = Indicates an estimated value.

Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)			Sample ID	30055/8255	30060/1625	30060/1627	31683/S1	31683/S2
			Sampling date / time	11-Mar-2022 00:00	11-Mar-2022 00:00	11-Mar-2022 00:00	11-Mar-2022 00:00	11-Mar-2022 00:00
Compound	CAS Number	LOR	Unit	ES2208971-001	ES2208971-002	ES2208971-003	ES2208971-004	ES2208971-005
				Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)								
pH Value	----	0.1	pH Unit	**5.9**	**6.4**	**6.3**	**5.2**	**5.4**
EA010: Conductivity (1:5)								
Electrical Conductivity @ 25°C	----	1	µS/cm	**27**	**25**	**20**	**64**	**125**
EA055: Moisture Content (Dried @ 105-110°C)								
Moisture Content	----	0.1	%	**21.6**	**25.8**	**17.5**	**31.3**	**31.0**
ED040S : Soluble Sulfate by ICPAES								
Sulfate as SO4 2-	14808-79-8	10	mg/kg	**30**	<10	<10	**50**	**110**
ED045G: Chloride by Discrete Analyser								
Chloride	16887-00-6	10	mg/kg	**----**	**----**	**----**	**50**	**120**

Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)			Sample ID	31683/S3	31683/S4	----	----	----
			Sampling date / time	11-Mar-2022 00:00	11-Mar-2022 00:00	----	----	----
Compound	CAS Number	LOR	Unit	ES2208971-006	ES2208971-007	--------	--------	--------
				Result	Result	----	----	----
EA002: pH 1:5 (Soils)								
pH Value	----	0.1	pH Unit	**4.9**	**5.0**	----	----	----
EA010: Conductivity (1:5)								
Electrical Conductivity @ 25°C	----	1	µS/cm	**158**	**143**	----	----	----
EA055: Moisture Content (Dried @ 105-110°C)								
Moisture Content	----	0.1	%	**29.5**	**30.2**	----	----	----
ED040S : Soluble Sulfate by ICPAES								
Sulfate as SO4 2-	14808-79-8	10	mg/kg	**80**	**110**	----	----	----
ED045G: Chloride by Discrete Analyser								
Chloride	16887-00-6	10	mg/kg	**200**	**120**	----	----	----